

### DESIGN APPROVAL

#### BURMA PIT ROAD LITTLE TONSINA BRIDGE REPLACEMENT

PROJECT NO. NSHWY00479

Requested by:

Lauren Little, P.E. Group Chief Northern Region

Design Approval Granted:

Sarah Schacher, P.E. Preconstruction Engineer Northern Region 10/5/2020

9/25/2

Date

Date

Distribution: NR Design Directive 20-01 Distribution

#### DESIGN STUDY REPORT FOR

#### BURMA PIT ROAD LITTLE TONSINA BRIDGE REPLACEMENT

PROJECT NO. NSHWY00479

# PREPARED BY: Travis Donovan, P.E.

### UNDER THE SUPERVISION OF: David Arvey, P.E.



ALASKA DEPARTMENT OF TRANSPORTATION AND PUBLIC FACILITIES NORTHERN REGION DESIGN AND ENGINEERING SERVICES SEPTEMBER, 2020

### BURMA PIT ROAD LITTLE TONSINA BRIDGE REPLACEMENT PROJECT NO, NSHWY00479 **Table of Contents**

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#### **INTRODUCTION/HISTORY**

The Alaska Department of Transportation and Public Facilities (DOT&PF) in partnership with the Copper River Watershed Project proposes to construct a new bridge and realigned road to replace two culverts on Burma Pit Road across the Little Tonsina River near Richardson Highway MP 74.2.

The existing gravel access road is used by the Alyeska Pipeline Corporation to access the pipeline, by the DOT&PF to access a material pit, and by homeowners in the area.

The size of the two existing culverts result in an obstruction to fish passage and are restrictive to flood passage and the natural stream conveyance. Hydrologic analysis of the crossing suggests that prior to the construction of the Richardson Highway, the confluence of the Little Tonsina with the main fork of the Tonsina River was 1.5 miles to the north, and highway construction likely cut off approximately 1 mile of the river length, straightening the river and causing a headcut to the travel upstream. Major flooding occurred in the area in 2006 and required an emergency repair contract to make necessary repairs.



Figure 1

#### **PROJECT DESCRIPTION**

The DOT&PF in partnership with the Copper River Watershed Project proposes to construct a new bridge over the Little Tonsina River, on Alyeska Pipeline Access Road 14APL4 (Burma Road) approximately 300 feet west of the Richardson Highway Milepost (MP) 74.4, and approximately 41 miles south of Glennallen, Alaska.

The current crossing at the Little Tonsina River site comprises two culverts (Bridge #568), approximately seven feet tall by eleven feet wide. The two culverts have been determined to be a likely fish passage barrier by the Alaska Department of Fish and Game (ADF&G) for excessive gradient, constriction ratio, and outlet and inlet perch. The existing culverts are frequently over topped with high stream flows during break-up and heavy fall precipitation and frequently get plugged with debris.

The new proposed crossing of the Little Tonsina River will be a concrete deck bulb tee single span bridge, 103 feet long by 27 feet 4 inches wide. The new crossing will create viable fish passage, better flood mitigation, and form a more natural stream.



Figure 2

#### **DESIGN STANDARDS**

Design standards and guidelines which apply to the Burma Pit Road Little Tonsina Bridge Replacement are contained in the following publications:

- *Guidelines for Geometric Design of Very Low-Volume Local Roads* (GDVLVLR). AASHTO, 2001.
- *A Policy on Geometric Design of Highways and Streets* ( "Green Book"). American Association of State Highway and Transportation Officials (AASHTO), 2011.
- *Alaska Highway Preconstruction Manual* (HPCM). State of Alaska Department of Transportation and Public Facilities.
- AASHTO LRFD Bridge Design Specifications, Ninth Edition. AASHTO, 2020.
- AASHTO Guide Specifications for LRFD Seismic Bridge Design, second edition, AASHTO, 2011.
- Alaska Traffic Manual (ATM), ADOT&PF, 2016.
- *Roadside Design Guide*, 4<sup>th</sup> edition, 2010, AASHTO.
- Alaska DOT&PF Standard Specifications for Highway Construction, 2020, ADOT&PF.

A Design Speed of 25 miles per hour was selected for this road in accordance with GDVLVR and HPCM guidance. The road serves as a materials site access and Alyeska Pipeline access.

The project will not require the information from the Design Designation process and will be designed to the approved design criteria. The Design Criteria worksheet is attached

#### **DESIGN EXCEPTIONS AND DESIGN WAIVERS**

There are no design waivers or design exceptions for this project. The design criteria and design designation waiver are attached in Appendix A.

#### **DESIGN ALTERNATIVES**

Replace the existing river crossing with a bridge or culverts.

#### PREFERRED DESIGN ALTERNATIVE

Replace existing culvert crossing with a new bridge structure.

In order to provide an adequate culvert design to accommodate fish passage, a 35'-4" span by 20' rise high profile arch with a 10' diameter overflow culvert would have to be constructed. This configuration would require an approximate 6 foot grade raise at the crossing. It is anticipated that a new culvert crossing would cost the same or more than a new bridge.

Benefits of the bridge allow for the existing crossing to remain in place to accommodate access to pipeline, area residents, and the material site from existing road during construction of the new bridge on new alignment. A new bridge is also more optimal for fish passage and flood

mitigation than a culvert configuration. For these reasons, a new alignment and bridge crossing was selected and the preferred design alternative. **3R ANALYSIS** 

Not applicable. This is a reconstruction project.

#### TRAFFIC ANALYSIS

No Traffic Analysis has been performed on this roadway.

#### HORIZONTAL/VERTICAL ALIGNMENT

The grades and all the horizontal and vertical curves for the project will meet AASHTO design standards for a design speed of 25 mph. The maximum grade on this project will be 5 percent, and the minimum radius of curvature will be 185 feet. See the attached project Design Criteria Worksheet and preliminary plan and profile sheets.

#### **TYPICAL SECTION(S)**

A new road alignment will be constructed for this project. The typical section will feature 10 ft wide lanes with 2 ft shoulders, 6 ft wide 4:1 foreslopes that hinge to a 2:1 foreslope/backslope to the fill/cut catch point. The new road section will consist of 6 inches of Surface Coarse Grading E-1 over a varying depth of Select Material Type A. The barn roof style embankment foreslopes allow for shallower recoverable slopes while also reducing the overall footprint of the new alignment.



#### Figure 3

#### **PAVEMENT DESIGN**

Not Applicable, finished project will be a gravel road.

#### PRELIMINARY BRIDGE LAYOUT

The new bridge will be a 27'-4"(total width), 24'(clear width), 103 ft. long single span prestressed concrete bulb-tee girder bridge. Preliminary bridge plans are in Appendix X.

#### **RIGHT-OF-WAY REQUIREMENTS**

The new road alignment and bridge will require ROW from Chugach Native Corporation. It is anticipated that the ROW needed will be donated or a land swap will be implemented. Existing road is a shared easement with Alyeska.

#### MAINTENANCE CONSIDERATIONS

Existing gravel road is currently maintained by DOT Maintenance. The realigned roadway will be comparable in length and width as the existing roadway.

The new bridge will be an added structure to DOT's inventory for biannual inspection. The bridge will allow for debris to pass freely under the bridge.

There will be a 6 inch layer of E-1 surface course added to the entire new alignment to help facilitate ease of maintenance.

#### **MATERIAL SOURCES**

Potential sources include:

#### <u>MS 71-1-014-5:</u>

Tonsina River Pit, Located ~MP 74.5 Richardson Highway - Potential source of Type A and B materials. Current knowledge indicates this site may be pretty well depleted. This site is authorized under a ROW grant on Chugach land. Pit use for anything other than material extraction will require coordination with Chugach. Pit expansion will most likely be required to provide materials to meet project requirements.

#### <u>MS 71-1-006-5:</u>

Burma Road Quarry Pit, Located ~3 miles down Burma Pit Rd accessed from MP 74.5 Richardson Highway – Potential source of Rip Rap. There is a current contract for this pit that expires December 2020. The renewal for this contract is being reviewed by the Director before it can be signed off on. Timeline for renewal is unknown.

#### MS 71-1-024-5: Tonsina Lodge Pit:

Located ~MP 78.5 Richardson Hwy. Potential source of Type A and B Materials. Site has previously produced concrete and paving aggregate as well as Base, Surface Coarse, and Subbase. Site has a current ROW grant and is non-expiring and available for use. Much of the material from this site is moisture sensitive and unstable due to the relatively high portion of the P200 consisting of clay-size particles. Site is not recommended for use on this project.

#### <u>MS 71-1-015-5:</u>

Located ~MP 67 of Richardson Hwy. Potential source of Type A and B Materials. Site has previously produced concrete and paving aggregate as well as Base, Surface Coarse, and Subbase. Site has a current ROW grant and is non-expiring and available for use.

#### MS 850-036-5:

Located ~MP 5 Edgerton Highway - Potential Source of Type A and B materials. DNR pit. Current material sale contract expires January 31, 2024.

#### **UTILITY RELOCATION & COORDINATION**

Existing overhead power owned by Copper Valley Electric Association (CVEA) and underground telecommunication owned by General Communication Inc, (GCI) utilities will be impacted by new road alignment and bridge and will need to be relocated prior to construction. The access road is used for Trans Alaska Oil Pipeline access and will require coordination with Alyeska. The project will have no impact on the Trans Alaska Oil Pipeline.

#### ACCESS CONTROL FEATURES

No access control features are included. Burma Pit Road is not an access-controlled facility.

#### **PEDESTRIAN/BICYCLE (ADA) PROVISIONS**

No separate facilities will be included for bicycle and pedestrian traffic. Pedestrians and bicycles will share the road with vehicular traffic.

#### SAFETY IMPROVEMENTS

There are no specific safety improvements proposed on this project.

#### INTELLIGENT TRANSPORTATION SYSTEM FEATURES

Not applicable. There are no intelligent transportation system features within the project limits.

#### DRAINAGE

The Little Tonsina is an anadromous river near Tonsina, Alaska. The Little Tonsina flows into the main fork of the Tonsina which is a tributary of the Copper River. The Copper River outlets into Prince William Sound near Cordova, Alaska. The crossing is comprised of two seven foot tall by eleven foot wide CMP squashed arch culverts. The culverts have been documented as being in poor condition. Flooding, overtopping, and debris clogs have been observed at this site.

The new bridge crossing will allow for the river to form a more natural stream that mimics the natural stream characteristics, and allow for the unrestricted movement of all fish and wildlife, nutrients, sediment and woody debris to the greatest extent possible.

#### **SOIL CONDITIONS**

The project site is located within the Continental Climate Zone of Alaska (Hartman and Johnson, 1984), characterized by short warm summers, long cold winters, and relatively low precipitation and humidity. Temperature extremes range from about -60 to 90 degrees Fahrenheit. The site receives as much as about 19.5 hours of sunlight in the summer (24 hours including twilight), and as little as about 5 hours in the winter.

The mean annual precipitation is 12.65 inches, the anticipated date of spring thaw is July 21 and fall freeze is August 21. (Western Regional Climate Center)

A foundation geology investigation was conducted in October of 2018 which consisted of one test hole in the proposed vicinity of the bridge structure abutment.

Groundwater was encountered in the test hole at 13.5 feet below the ground surface.

Permafrost or seasonally frozen ground was not observed in the test hole. Previous explorations in adjacent areas encountered permafrost in the shallow subsurface. If construction occurs during winter months, seasonally frozen ground could be expected at the site.

The test hole indicated approximately 13 feet of embankment fill consisting of silty sand with gravel, cobbles and boulders. Underlying the embankment material was silty gravel or silty sand material to a depth of 47.5 feet, transitioning to a coarse gravelly clay or clayey gravel to gravel with silt to 80 feet depth. Drilling indicated significant oversized material content, including large boulders. Underlying the coarse deposits, the test hole penetrated silt, silty sand and clayey sand to the hole bottom at 101.5 feet. Bedrock was not encountered in the test hole.

#### **EROSION AND SEDIMENT CONTROL**

No Extraordinary conditions.

The contractor will develop a Storm Water Pollution Prevention Plan (SWPPP) based on the Alaska Storm Water Pollution Prevention Plan Guide. BMP's will be implemented to stabilize erodible soils.

The area of ground disturbance for this project is approximately 0.1 acres below ordinary high water.

#### **ENVIRONMENTAL COMMITMENTS**

State Funded Categorical Exclusion. Currently, project is not FHWA funded but has the potential to be. Environmental Document will be finalized once final funding sources are determined.

#### WORK ZONE TRAFFIC CONTROL

This project is not considered significant for traffic control per DOT&PF's Policy and Procedure 05.05.015.

During construction of the new bridge and portions of the new alignment, traffic will be maintained on the existing alignment. Some portions of the work may require intermittent lane closures and/or reduction of traveled way widths. The Contractor will develop traffic control plans for the work that will be submitted to the Department for approval prior to implementation.

#### VALUE ENGINEERING

Not applicable. The project does not meet the criteria for a Value Engineering study.

#### COST ESTIMATE

The estimated costs for this project are as follows:

Design	\$297,000
Utilities	\$150,000
Right of Way	\$0.00
Construction (Includes 20% Engineering)	\$2,141,725.00
Total Cost of Project	\$2,588,725.00

Total Cost of Project

\$2,588,725.00

### **APPENDIX A**

DESIGN CRITERIA AND DESIGN DESIGNATION

## MEMORANDUM

TO: Sarah E. Schacher, P.E. Preconstruction Engineer Northern Region

**THRU:** Lauren Little, P.E. Croup Chief Northern Region

FROM: David Arvey, P.E.

Engineering Manager Northern Region

### **State of Alaska** Department of Transportation & Public Facilities Northern Region Design and Engineering Services

**DATE:** July 20, 2020

FILE NO: H:\Projects\Rich\_Hwy\00479\_Little\_Tonsina\Design\Design Criteria\DesignDesignationWaiverMemo

PHONE NO: (907) 451-5371

FAX NO: (907) 451-5126

SUBJECT: Burma Pit Road Little Tonsina Bridge Replacement NSHWY00479 Design Designation Waiver

A waiver of the Highway Preconstruction Manual requirement for a Design Designation (HPM 1100.4.1) is requested.

The purpose of this project is to realign Burma Pit Road to construct a new bridge over the Little Tonsina River to replace two undersized culverts.

The project design will not require the information from the Design Designation process and will be designed to the approved design criteria.

Approved: \_\_\_\_\_

7/20/2020

rah E. Schacher, P.E., Preconstruction Engineer

Date

TAD/

cc: Scott Vockeroth, NR Planning

#### ALASKA DOT&PF PRECONSTRUCTION MANUAL Chapter 11 - Design PROJECT DESIGN CRITERIA

Project Name: Burma Pit Road Little To	nsina Bridge Replace	ement			
✓ New Construction/Reconstruction	3R	PM	Other:		
Project Number: NSHWY00479				NH	HS Von NHS
Functional Classification:	Rural Local				
Design Year:	2040		Present ADT:		Less than 400 vpd
Design Year ADT:	Less than 400 vpd		Mid Design Period A	NDT:	n/a
DHV:	n/a		Directional Split:		50/50
Percent Trucks:	n/a		Equivalent Axle Loa	ding:	n/a
Pavement Design Year:	n/a		Design Vehicle:		WB-40
Terrain:	Level		Number of Roadway	/s:	1
Design Speed:	25				
Width of Traveled Way:	20				
Width of Shoulders:	Outside:	2		Inside:	n/a
Cross Slope:	3%				
Superelevation Rate:	6% max				
Minimum Radius of Curvature:	185	(GDVLVI	LR Exhibit 3)		
Min. K-Value for Vert. Curves:	Sag:	17 (GB 0	Ch.5 Table 5-3)	Crest:	7 (GDVLVLR Exhibit 12)
Maximum Allowable Grade:	7% (6% Bridge)	(GB Ch.	5 Table 5-2)		
Minimum Allowable Grade:	0% (0.5% Bridge)				
Stopping Sight Distance:	115 ft	(GDVLV	LR Exhibit 8)		
Lateral Offset to Obstruction:	2 ft Minimum				
Vertical Clearance:	20 ft min	Power li	nes will cross new al	ignment	
Bridge Width:	24 ft (Clear Width)				
Bridge Structural Capacity:	HL-93	(AASHT	O LRFD Bridge Desig	In Specifications)	
Passing Sight Distance:	450 ft	(GB Ch.	3 Table 3-4)		
Surface Treatment:	T/W:	Gravel		Shoulders:	Gravel
Side Slope Ratios:	Foreslopes:	4:1 (clea	r zone), 2:1	Backslopes:	2:1
Degree of Access Control:	DW Entrance Regul	ations			
Median Treatment:	N/A				
Illumination:	N/A				
Curb Usage and Type:	N/A				
Bicycle Provisions:	Shared Roadway				
Pedestrian Provisions:	Shared Roadway				
Misc. Criteria:	N/A				

Proposed - Designer/Consultant: **Endorsed - Engineering Manager:** 

Approved - Preconstruction Engineer:

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Date:	7	15	20
Date:	7/	171	20
Date:	7/20/2020		

Shaded criteria are commonly referred to as the *FWHA 13 controlling criteria*. For NHS routes only, these criteria must meet the minimums established in the Green Book (*AASHTO A Policy on Geometric Design of Highways and Streets*). For all other routes, these criteria must meet the minimums established in the *Alaska Highway Preconstruction Manual*. Otherwise a Design Exception must be approved.

Design Criteria marked with a " # " do not meet minimums and must have a Design Exception(s) and/or Design Waiver(s) approved. See the Design Study Report for Design Exception/Design Waiver approval(s) and approved design criteria values.

#### **APPENDIX B**

**ENVIRONMENTAL DOCUMENT** (only include the signature page of the FONSI or ROD)

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APPENDIX C

PRELIMINARY PLAN AND PROFILE SHEETS



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NO.	DATE	REVISION	STATE	PROJECT DESIGNATION	YEAR	SHEET	TOTAL
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**APPENDIX D** 

PRELIMINARY BRIDGE PLANS



STATE	PROJECT DESIGNATION	YEAR	SHEET NO,	TOTAL SHEETS
ALASKA	NFHWY00479	2021	N1	N14

BRIDGE DRAWING INDEX	
TITLE	DWG, NO,
GENERAL LAYOUT	
SITE PLAN	2
RIPRAP LAYOUT	3
RIPRAP DETAILS	4
ABUTMENT I	5
ABUTMENT 2	6
WINGWALLS	7
FRAMING PLAN AND TYPICAL SECTION	8
GIRDERS	9
STEEL BRIDGE RAILING, 3-TUBE	10
TRANSITION RAIL, 3-TUBE	
TEST HOLE LOGS AND LOCATIONS	12-14



STATE	PROJECT DESIGNATION	YEAR	NO,	SHEETS
ALASKA	NFHWY00479	2021	N2	N14

#### GENERAL NOTES

AASHTO LRFD Bridge Design Specifications, 2020 Edition, with latest interim specifications.

Seismic design per AASHTO Guide Specifications for LRFD Seismic Bridge Design, 2011 with latest interim revisions.

Includes 50 psf for all wearing surfaces.

= 0.35 = 0.79 = 0.30 Site Class = C Liquefaction Potential = Low AASHTO 7% probability of exceedance in 75 years.

- ASTM A706, Grade 60, Fy = 60,000 psi ASTM A970 Headed bars, Class HA. Space reinforcement evenly unless otherwise noted.
- . See "GIRDERS" Dwg.
- .Class A Concrete unless otherwise noted, f'c = 4,000 psi
- .ASTM A709, Grade 36T3, Fy = 36,000 psi Galvanize structural steel in accordance with AASHTO M111 unless noted otherwise.
- Pile Tip reinforcing is required.

#### PILE DATA TABLE

11	LL DATA	TADLL			
	DRIVING CRITERIA			DESIGN DATA	
M TION	ESTIMATED PILE TIP ELEVATION (ft)	DRIVING RESISTANCE (K)	STRENGTH I FACTORED LOAD (K)	NOMINAL RESISTANCE (K)	$\begin{array}{l} \textbf{RESISTANCE} \\ \textbf{FACTOR,}  \varphi \end{array}$
	1,591	445	290	445	0.65
	1,591	445	290	445	0.65

vay	n.f.	= near face
r	No.	= number
	0.C.	= on center
air face	0.H.W.	= ordinary high water
ed concrete	OE	= overhead electrical line
essive strength	pcf	= pounds per cubic foot
ad concrete	psf	= pounds per square foot
ed concrete	, psi	= pounds per sauare inch
essive strength at release	R	= radius
4	R.O.W.	= right of way
	RT	= right
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oounds per square inch	$\gamma p$ .	= lypicul
3		= undergound telephone
foot	01	= untrasonic testing
sum	VPC	= point of vertical curve
	VP1	= point of vertical intersection
um	VPI	= point of vertical tangent
ım	w/	= with
nically stabilized earth		



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	QUANTITIES BY: Designer CH	HECKED: Checker		3132 Channel Drive Juneau, Alaska 99801 907-465-2975	RII

d\568\568-RR LAYOUT Fri, Jun/05/20 08:39am

STATE	PROJECT DESIGNATION	YEAR	SHEET NO,	TOTAL SHEETS
ALASKA	NFHWY00479	2021	Ν3	N14

TONSINA BRIDGE BURMA ROAD PRAP LAYOUT



DESIGNED BY:	Designer	CHECKED:	Checker		STATE OF ALASKA	ז זיייד ד
DRAWN BY:	Sam Sollie	CHECKED:	Designer		AND PUBLIC FACILITIES	
				PRELIMINART PLAN	BRIDGE SECTION 3132 Channel Drive	
QUANTITIES BY:	Designer	CHECKED:	Checker		Juneau, Alaska 99801 907-465-2975	RIF

STATE	PROJECT DESIGNATION	YEAR	SHEET NO,	TOTAL SHEETS
ALASKA	NFHWY00479	2021	N4	N14

TONSINA BRIDGE BURMA ROAD PRAP DETAILS





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	W001			10' 6"		TAIT		
	W907 W902	9	4 20	VARIES			<u></u>	2
	W903	9	4	16'-1"	Bi	ENT		<b>.</b>
	C403	E 4	8	3'-10" 7'-7"	BI	ENT		2-2
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		TYPICAL	TEST HOLE LOG		Hole diar	neter —		NOTES:		
Elevation(ft) 100 -		DATE: Da STATION /	te begun – Date completed ′OFFSET: XX+XX FEET RT or LT			3.5	5 in Depth(ft)	1) The test hole logs logs, based on pos	depicted graphically in st-field investigation rev	these drawings are distillations iew and analysis. These drafte
-	Frozen	- /,	—Graphic materials description _Stratum contact				- - 	made to field desc observations of roc drafted loas.	riptions based upon lab ck and soil sampled dur	oratory test data, review and a ing the drilling program are not
-			-Estimated stratum contact				-	2) Description of soils	follows Alaska Geotech	nical Procedures manual.
<i>95 -</i> -	Observed							Classification of so	ils follows Unified Soil (	Nassification System (ASTM D24
-	Ground {		Soil graphic and soil type explanation	<u>n</u>			-	3) The test hole logs	from these sheets are	an integral part of the Foundat
-	Water `` Date:	202	GRAVEL (GP)		SILTY GRAVEL (	(GM)	-	Bid Documents – i investigation is con	invitation to bid/notice ntained in the report.	to bidders. Important informat The test hole loas are not seve
- 90 -	<u>xx/xx/xx</u>		GRAVEL (GW)	<del>ارم</del> الم	CLAYEY GRAVEL	. (GC)	-	correctly interprete	d without reference to	the Foundation Geology Report.
-			SAND (SP)		SILTY SAND (SI	1)	-			
-	4	••••	SAND (SW)		CLAYEY SAND (	sc)	-			
-			SILT (ML)		ORGANIC SILTS	(01)	-			
85 -		$\mathcal{H}$			OPCANIC CLAYS		-			
-					UNUANIC CLATE		-			
-			CLAY (CL)				-			
- 80 -			ORCANICS OR PEAT (PT)				-			
-			COPPLE OF POLLOEP INDICATED BY		CTION OF COPE		-			
-	1		COBBLE OR BOOLDER INDICATED BT	DRILL READ	CHON OR CORE		-			
-	thoc		ICE				-			
<i>75</i> -	Met		WEATHERED BEDROCK (Strength Grad	de, Weathei	ring Grade)		-			
-	bu.		BEDROCK (Strength Grade, Weatherin	ng Grade)			-			
-	Drill		COAL				-			
-			RQD % = Sum of lengths of core p	ieces > 4"	/ total length of	run	-			
70 - -			L = Longest length of core in run				-			
-			S = Shortest lkength of core in run	•/			— <i>32.0ft</i>			
-			SAND with Silt $\rightarrow$ USCS solution $200 = 8\%$ $\rightarrow$ $\%$ passin	i name a the #200	0 (0.075mm) sieve		-			
- 65 -			Sa=42% % sand,	retained or	n #200 (0.075mm)	sieve	-			
-			and p	assing #4	(4.75mm) sieve		-			
-			Gr=50% % gravel,	retained o	on #4 (4.75mm) si	eve	-			
-	ŧ		Ora=10% - moisture	content			-			
60 -			PI=8	index			-		т	YPICAL PENETROMETER TE
-			LL=18 - liquid lim	it			-		<u> </u>	
-			SM USCS cla PP=2.0 pocket p	ssification enetromete	er (ton / SE)		-		D. S	ATE: Date begun – Date comp TATION / OFFSET: XX+XX / R
- 55 -			$TV=2.0$ $\rightarrow$ torvane	(ton / SF)	)		-			
-			GRAPHICS: (double symbols with sp	lit graphic:	s may be used to	indicate	-			0 100 200 300
-			combinations of soil t	vpes)			-		Г	
-			07–3533 = soil sample number (ye	ar – samp	ole number)					
50 -			SNT = Sample Not Tested		and and third of the		-		Q	
-			standard penetration test sampler	vs for secc w/ 1.4 ID,	ona ana tnira o in 2" O.D. using a	CME	-		ho	$\left \right\rangle$
- In	terval sampled with		autohammer with 140 lb. hammer	and a 30"	" freefall latest edit	ion AASHTO	-		test set	
- 45 -	recovery shaded		- 7 206 (ASTM D1586).				-		t s	
-			SP160 = same as SP1 except, inst method was used.	ead of CM	E autohammer, the	e cathead/rope	-		tt o	
-			SS = blow count with 2" I.D., 2.5"	, O.D. sami	pler driven bv a 34	O Ib. CME	-		Vati Dep	
-			autohammer with a 30" freefall.	,			-		Ele Dd 1	
40 -		ORE	$MC = blow \ count \ with \ 2.5" \ I.D., \ 3$	" O.D. sam	pler driven by a 3.	40 Ib. CME	-		מ ע	2
-			autohammer with a 30° freefall.				-		Srou	
-			—Indicates no Valia SP1		, , ., "		-			
- 35 -		60	Grab sample from arill cuttings or a	irectly from	m noie siaewali		-			
-				defined as	s 50 or more blow	s per 6"	-			
-		*-	-{ increment, 100 total blows, or no	movement	observed with 10 s	successive	-			
-			blows.				-		Bo	ottom of hole (BOH)
30 -		<i>vs</i> -	-Vane shear test, undrained shear st	rength, PS	F		-			<u>NOTES:</u>
		57 -	— Thin walled sampler, pushed				70.0ft			Penetrometer W/2.5" (
		Bottom of	hole (BOH)		Toto	depth				Hammer using a 340 .
DESIGNED BY:	Designed CHECKED:	Engir	neer							፣ ፣ጥጥ፣ י
	Orafter area					:	STATE OF A	LASKA		
DRAWN BY:	CHECKED:	Engli			[]	DEPARTMEN	NT OF T	RANSPORTATION		
						AND	PUBLIC ]	FACILITIES		
QUANTITIES BY:	Lagineer   CHECKED:	Engir				ST	ATEWIDE MA	TERIALS		



<u>NOTES:</u> Penetrometer W/2.5" O.D., with a CME AUTOMATIC Hammer using a 340 lb. weight and a 30" freefall



DATE: Date begun – Date completed STATION / OFFSET: XX+XX / RT or LT (feet)

	STATE	PROJECT DESIGNATION	YEAR	SHEET NO.	TOTAL SHEETS
al field	ALASKA	NSHWY00479	2020	N13	N14

ole logs depicted graphically in these drawings are distillations of the origina ed on post-field investigation review and analysis. These drafted logs include changes field descriptions based upon laboratory test data, review and analysis. Detailed field ns of rock and soil sampled during the drilling program are not reproduced in the

ion of soils follows Unified Soil Classification System (ASTM D2487).

ole logs from these sheets are an integral part of the Foundation Geology Report. See Construction Contract nents — invitation to bid/notice to bidders. Important information about the test hole logs and the foundation ion is contained in the report. The test hole logs are not severable from and cannot be completely and

TYPICAL PENETROMETER TEST LOG

BLC	OWS / F	ООТ			Hole	e diameter	$\overline{}$
400	500	600	700	800	900	1000	$\succ$
						<b>I</b>	2.5 in.

Practical refusal with penetrometer test-

## LITTLE TONSINA BRIDGE

BURMA ROAD

**TEST HOLE & PENETROMETER LEGEND** 



BRIDGE NO. 0568 <u>DWG. NO.</u> 13



~						
	DESIGNED BY:	Designed	CHECKED: Engineer			LITT
	DRAWN BY:	Drafter	CHECKED: Engineer	DEPARTMENT OF TRANSPORTATION AND PUBLIC FACILITIES		
	QUANTITIES BY:	Engineer	CHECKED: Engineer	STATEWIDE MATERIALS	TEST	HOLI

STATE	PROJECT DESIGNATION	YEAR	SHEET NO.	TOTAL SHEETS
ALASKA	NSHWY00479	2020	N14	N14

'LE TONSINA BRIDGE

BURMA ROAD

E & PENETROMETER LOGS



bridge no. 0568 dwg. no. **14**